# **Kingston Intersection Study I-587 AT ALBANY AVENUE/BROADWAY INTERSECTION** 32 28 587



**Conceptual Design Report - Appendices Technical Memorandum** 



February 201



### City of Kingston I-587/Albany/Broadway Intersection Study **Conceptual Design Report - Appendices**

## **Prepared for the Ulster County Transportation Council**

Prepared by:

Fitzgerald & Halliday, Inc.

In association with:

AECOM **Alternate Street Design** URS

February 2011



#### Appendix A: Design Workshop – Stakeholder Interview Comments

#### Major Themes - What We Heard

- 1. Traffic congestion and safety
  - a. Prefer constrained environment that does not sacrifice qualities of place to maximize volume and speed of traffic
  - b. Its OK if traffic moves slowly as long as it moves smoothly
  - c. Fix hazardous conditions for pedestrians and turning movements for cars; especially Clinton at Albany
  - d. Fix double-left turn from Broadway onto Albany
  - e. Fix unsafe merges; Maiden lane at Albany; East St James at Albany
  - f. Eliminate need for unsafe U-turns to turn east to get to I-587 from Albany Ave west of the intersection; to turn west to get to I-587 from Albany Ave east of the intersection
  - g. Eliminate unsafe left turns
  - h. Roundabout is good solution if it works for all vehicles and travelers whether on foot, in car or on a bus
  - i. Create access to Mall from I-587
  - j. Don't think roundabout would work; Concern roundabout would not accommodate large buses
  - k. Local street network is not being used due to Broadway configuration today
  - I. Some drivers and fire trucks/ambulances avoid the intersection and Albany Avenue to go Uptown use local streets instead
  - m. Employ traffic calming; can park be used to encourage traffic to slow down?; traffic calming especially needed on Broadway

- n. Traffic calming should not include speed humps they are a problem for fire trucks, buses, and snowplows
- Driveways poorly located hazardous to get into and out of; driveway at Albany and Clinton; driveway at Broadway and St James
- p. Downgrade I-587 to local street (two lanes) before it reaches the intersection
- Question of the street are a problem need alternate internal connections formalized
- r. Access to/from St James too difficult it functions as a one-way street
- s. Parking not well located is more of an issue in Stockade and Uptown
- 2. Signage
  - a. Fix signage
  - b. There is too much or too little too late
  - c. Some signage is incorrect
  - d. Some signage is in the wrong places; it can be confusing
  - e. Mass of signage is ugly blocks views
- 3. Community character/Revitalization
  - a. Make this a destination a neighborhood place; the intersection is like the handle in the middle of a barbell .....it is the connector between neighborhoods but the handle/connection is broken
  - b. Make this an aesthetic gateway with cohesive theme (branding)– Gateway is critical
  - c. The intersection creates a barrier between neighborhoods
  - d. Sense of a place here is critical
  - e. Restore village green at the intersection
  - f. Intersection should be a gateway not a highway
  - g. Huge redevelopment opportunity on the area particularly Broadway at St James





### City of Kingston I-587/Albany/Broadway Intersection Study

#### Conceptual Design Report

- h. Need to improve aesthetics of street frontage building facades as well as building orientation to street
- i. Keep green space and Dinosaur
- j. Economic development issues tied to many factors including property tax burden for business – may inhibit incentives for new business to locate in intersection area if it becomes a more people oriented space
- k. Business like 721 Media could be anchor for revitalizing area
- 4. Bicycles, Pedestrians, and Transit
  - a. Make it easier for large vehicles to navigate the intersection
  - b. Create complete streets –give some priority to bicycles and pedestrians
    - i. Fix pedestrian crossings locations, visibility, speeds of traffic, crossing signals, and pedestrian phase on traffic signal
    - ii. Improve transit access points bus stop on Academy Green needs safe pull off
    - iii. Add bike lane on Broadway
  - c. Make it safe to walk from one side of the intersection to the other
  - d. Concern about safety of cut-through traffic in the neighborhoods conflict with people out using the streets to walk, bike, socialize
  - e. Rail bed might be better used for pedestrian system
  - f. Would favor using rail bed for local/scenic rail service from Rondout to Stockade
- 5. Emergency services
  - Fire trucks/ambulances avoid the intersection use local streets instead- to avoid bottleneck and stopped traffic
  - b. Fire houses on either side of intersection forced to pass through the area to get to emergency; avoid the

intersection as it slows down or stops emergency services vehicles; need pre-emption signal

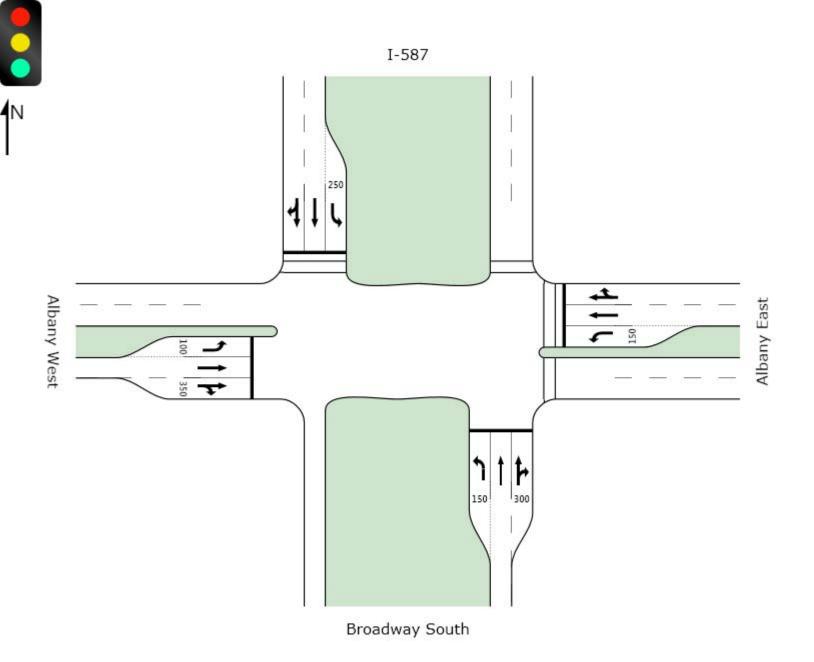
- Police regularly use intersection (pass through) to respond to incidents – there are lots of accidents in the intersection
- d. Satellite public safety complex at/near the intersection might be nice but ability to provide coverage constrained by size of the police force
- 6. Open spaces, Public spaces, and the Natural Environment
  - a. Preserve rail bed for potential rail trail multi-use path connections
  - b. Eventually connect rail trail multi-use path with on-street network to destinations shopping plaza
  - c. Build complete streets with public spaces/green spaces
  - d. Preserve Academy green and improve access to it
  - e. Preserve historic structures protect them
  - f. Be aware of flood storage character of area on either side of 587; levee used to avoid flooding there but floodplain/some flooding persists
  - g. Be aware of wetlands and floodplain along Esopus Creek





February 2011

Appendix B: Analysis Summaries



Albany and I-587 PM Peak SIgnals existing signals Signals - Actuated Cycle Time = 87 seconds

Movem	ient Pei	formance - V	/ehicles								
		Demand	1.0.7	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Coutby D		veh/h	%	v/c	sec		veh	ft		per veh	mph
South: E											
3L	L	228	2.0	0.676	36.1	LOS D	10.3	262.8	0.79	0.78	19.7
8T	Т	276	2.0	0.322	32.5	LOS C	6.9	174.7	0.85	0.70	19.5
8R	R	1	2.0	0.343	41.8	LOS D	6.7	170.5	0.85	0.88	18.9
Approac	ch	505	2.0	0.676	34.1	LOS C	10.3	262.8	0.83	0.74	19.6
East: Alt	bany Eas	st									
1L	L	57	2.0	0.233	31.5	LOS C	2.8	70.4	0.72	0.73	21.1
6T	Т	517	2.0	0.528	20.4	LOS C	15.2	385.8	0.78	0.68	23.5
6R	R	284	2.0	0.527	21.2	LOS C	11.9	302.5	0.73	0.85	25.1
Approac	ch	858	2.0	0.528	21.4	LOS C	15.2	385.8	0.76	0.74	23.8
North: I-	587										
7L	L	253	2.0	0.672	46.9	LOS D	12.7	322.5	0.93	0.82	17.1
4T	Т	234	2.0	0.546	35.2	LOS D	11.4	288.9	0.91	0.76	18.8
4R	R	76	2.0	0.129	9.3	LOS A	0.5	11.6	0.10	0.70	31.4
Approac	h	563	2.0	0.672	37.0	LOS D	12.7	322.5	0.81	0.78	18.9
West: Al	lbany We	est									
5L	Ĺ	95	2.0	0.612	45.4	LOS D	5.5	140.4	0.85	0.79	17.4
2T	Т	507	2.0	0.372	20.5	LOS C	10.6	268.4	0.72	0.61	23.8
2R	R	1	2.0	0.386	29.0	LOS C	8.6	218.4	0.70	0.94	22.8
Approac	ch	602	2.0	0.612	24.4	LOS C	10.6	268.4	0.74	0.64	22.5
All Vehic	cles	2528	2.0	0.676	28.1	LOS C	15.2	385.8	0.78	0.72	21.4

Level of Service (Aver. Int. Delay): LOS C. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS D. LOS Method for individual vehicle movements: Delay (HCM). Approach LOS values are based on average delay for all vehicle movements.

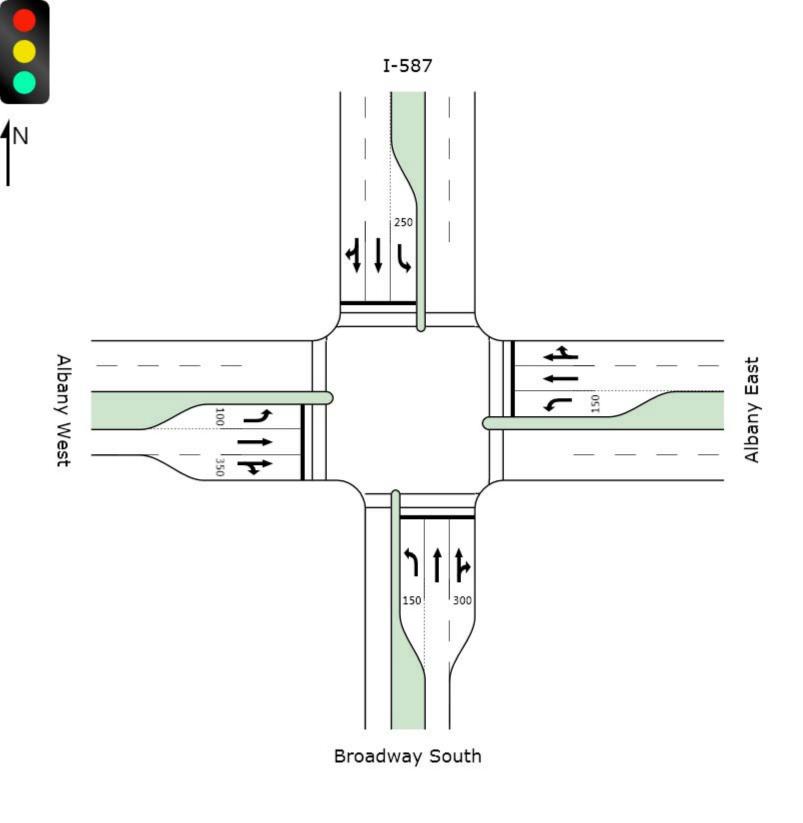
Mover	Movement Performance - Pedestrians										
Mov ID	Description	Demand	Average		Average Back		Prop.	Effective			
	Description	Flow ped/h	Delay sec	Service	Pedestrian ped	Distance ft	Queued	Stop Rate per ped			
P3	Across E approach	5	36.5	LOS D	0.0	0.1	0.90	0.90			
P5	Across N approach	5	38.4	LOS D	0.0	0.1	0.90	0.90			
All Pede	All Pedestrians		37.4				0.90	0.90			

Level of Service (Aver. Int. Delay): LOS D. Based on average delay for all pedestrian movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS D. LOS Method for individual pedestrian movements: Delay (HCM).

Processed: Monday, October 04, 2010 3:15:27 PM SIDRA INTERSECTION 5.0.2.1437 Project: \\.psf\Project files\Kingston, NY\Analyses\Kingston Albany.sip 8000067, ALTERNATE STREET DESIGN PA, SINGLE

Copyright © 2000-2010 Akcelik & Associates Pty Ltd www.sidrasolutions.com n Albany.sip





Albany and I-587 Option compact signalized intersection PM Peak Signals Signals - Actuated Cycle Time = 71 seconds

Mover	nent P <u>er</u>	formance -	Vehicles_								
		Demand		Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
O availle a	Dura a di vari	veh/h	%	v/c	sec		veh	ft		per veh	mph
	Broadway			0 700	44.0	100.0		0447	0.05	0.04	40.4
3L	L	228	2.0	0.733	41.2	LOS D	9.6	244.7	0.85	0.81	18.1
8T	Т	276	2.0	0.328	26.9	LOS C	5.9	149.6	0.86	0.70	21.2
8R	R	1	2.0	0.301	35.9	LOS D	5.7	145.2	0.85	0.87	20.6
Approac	ch	505	2.0	0.733	33.4	LOS C	9.6	244.7	0.85	0.75	19.7
East: All	bany Eas	st									
1L	L	57	2.0	0.226	35.5	LOS D	2.7	67.9	0.83	0.76	19.6
6T	Т	517	2.0	0.638	22.7	LOS C	14.3	363.1	0.87	0.75	22.4
6R	R	284	2.0	0.638	23.2	LOS C	11.8	300.4	0.83	0.85	24.3
Approac	ch	858	2.0	0.638	23.7	LOS C	14.3	363.1	0.86	0.79	22.8
North: I-	-587										
7L	L	253	2.0	0.546	34.4	LOS C	10.0	253.4	0.87	0.81	20.0
4T	Т	234	2.0	0.557	29.8	LOS C	9.8	249.8	0.91	0.76	20.3
4R	R	76	2.0	0.114	14.4	LOS B	1.7	43.7	0.44	0.74	28.0
Approac	ch	563	2.0	0.557	29.8	LOS C	10.0	253.4	0.83	0.78	20.9
West: A	lbany We	est									
5L	L	95	2.0	0.459	35.7	LOS D	4.3	109.7	0.83	0.76	19.6
2T	Т	507	2.0	0.419	21.3	LOS C	9.2	233.5	0.81	0.68	23.3
2R	R	1	2.0	0.408	29.8	LOS C	9.1	229.9	0.81	0.90	22.7
Approac	ch	602	2.0	0.459	23.6	LOS C	9.2	233.5	0.81	0.69	22.6
All Vehic	cles	2528	2.0	0.733	27.0	LOS C	14.3	363.1	0.84	0.75	21.6

Level of Service (Aver. Int. Delay): LOS C. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS D. LOS Method for individual vehicle movements: Delay (HCM). Approach LOS values are based on average delay for all vehicle movements.

Moven	Movement Performance - Pedestrians										
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance ft	Prop. Queued	Effective Stop Rate per ped			
P1	Across S approach	5	27.1	LOS C	0.0	0.0	0.87	0.87			
P3	Across E approach	5	30.5	LOS D	0.0	0.0	0.88	0.88			
P5	Across N approach	5	27.1	LOS C	0.0	0.0	0.87	0.87			
P7	Across W approach	54	30.5	LOS D	0.2	0.5	0.88	0.88			
All Pedestrians		69	30.0				0.88	0.88			

Level of Service (Aver. Int. Delay): LOS D. Based on average delay for all pedestrian movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS D. LOS Method for individual pedestrian movements: Delay (HCM).



Albany and I-587 Option compact signalized intersection PM Peak Signals No High-speed Broadway bypass. Signals - Actuated Cycle Time = 76 seconds

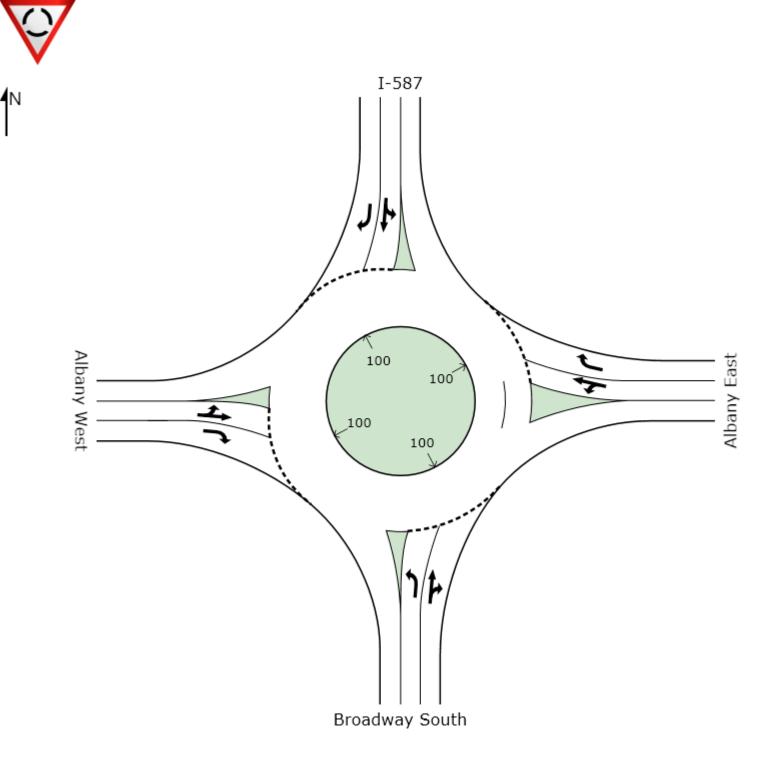
Movem	ent Pe	rformance - V	/ehicles								
May ID	T	Demand		Deg.	Average	Level of	95% Back (		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South: B	Broadwa	veh/h	%	v/c	sec	_	veh	ft	_	per veh	mph
3L	L	228	2.0	0.792	49.6	LOS D	10.7	270.5	0.87	0.83	16.3
8T	Т	276	2.0	0.298	33.3	LOS C	13.2	336.4	0.83	0.67	19.4
8R	R	1	2.0	0.281	48.8	LOS D	13.2	336.4	0.83	0.87	17.2
Approac		505	2.0	0.792	40.7	LOS D	13.2	336.4	0.85	0.75	17.2
			2.0	0.752	40.7	LOOD	10.2	000.4	0.00	0.75	17.0
East: Alb	bany Ea										
1L	L	57	2.0	0.386	45.8	LOS D	3.3	83.4	0.92	0.77	17.1
6T	Т	517	2.0	0.632	23.8	LOS C	15.0	381.9	0.87	0.75	22.0
6R	R	284	2.0	0.632	24.2	LOS C	12.5	318.4	0.83	0.85	23.8
Approac	h	858	2.0	0.632	25.4	LOS C	15.0	381.9	0.86	0.79	22.1
North: I-	587										
7L	L	253	2.0	0.585	38.2	LOS D	10.8	274.7	0.89	0.82	18.9
4T	Т	234	2.0	0.502	28.9	LOS C	10.0	253.1	0.88	0.74	20.6
4R	R	76	2.0	0.111	14.9	LOS B	1.8	46.9	0.44	0.74	27.8
Approac	h	563	2.0	0.585	31.2	LOS C	10.8	274.7	0.83	0.77	20.5
West: Al	bany W	est									
5L	L	95	2.0	0.470	35.7	LOS D	4.4	112.8	0.81	0.75	19.6
2T	Т	507	2.0	0.659	25.1	LOS C	15.8	401.6	0.87	0.75	21.6
2R	R	363	2.0	0.659	24.5	LOS C	14.4	365.2	0.77	0.85	23.6
Approac	h	964	2.0	0.659	25.9	LOS C	15.8	401.6	0.83	0.79	22.1
All Vehic	cles	2890	2.0	0.792	29.4	LOS C	15.8	401.6	0.84	0.78	20.9

Level of Service (Aver. Int. Delay): LOS C. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS D. LOS Method for individual vehicle movements: Delay (HCM). Approach LOS values are based on average delay for all vehicle movements.

Movement Performance - Pedestrians										
	5	Demand	Average	Level of	Average Back		Prop.	Effective		
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate		
		ped/h	sec		ped	ft		per ped		
P1	Across S approach	5	29.5	LOS C	0.0	0.0	0.88	0.88		
P3	Across E approach	5	31.5	LOS D	0.0	0.0	0.89	0.89		
P5	Across N approach	5	29.5	LOS C	0.0	0.0	0.88	0.88		
P7	Across W approach	54	31.5	LOS D	0.2	0.5	0.89	0.89		
All Pedestrians		69	31.2				0.89	0.89		

Level of Service (Aver. Int. Delay): LOS D. Based on average delay for all pedestrian movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS D. LOS Method for individual pedestrian movements: Delay (HCM).





Albany and I-587 Option 1 Right/through and left only lanes from I-587 PM Peak Roundabout

Site:	<b>Kingston</b>	PM	Option1

Movem	ent Pe	rformance - V	/ehicles								
	-	Demand	1.15.7	Deg.	Average	Level of	95% Back o		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South: E	roadwa	veh/h	%	v/c	sec	_	veh	ft		per veh	mph
300011. E		228	2.0	0.411	19.8	LOS B	3.5	88.9	0.87	0.99	26.1
	L		2.0								
8T	T	276	2.0	0.466	12.1	LOS B	4.6	116.2	0.90	0.96	29.5
8R	R	37	2.0	0.468	13.7	LOS B	4.6	116.2	0.90	0.98	29.3
Approac	h	541	2.0	0.466	15.5	LOS B	4.6	116.2	0.89	0.97	27.9
East: Alt	bany Ea	st									
1L	L	57	2.0	0.665	16.3	LOS B	6.0	152.1	0.76	1.06	28.7
6T	Т	517	2.0	0.667	8.9	LOS A	6.0	152.1	0.76	0.83	30.9
6R	R	284	2.0	0.415	9.3	LOS A	2.5	63.2	0.63	0.78	31.0
Approac	h	858	2.0	0.666	9.5	LOS B	6.0	152.1	0.72	0.83	30.8
North: I-	587										
7L	L	253	2.0	0.420	19.0	LOS B	3.6	91.9	0.85	0.97	26.5
4T	Т	234	2.0	0.437	10.8	LOS B	4.1	103.3	0.87	0.91	30.2
4R	R	76	2.0	0.437	12.4	LOS B	4.1	103.3	0.87	0.93	30.0
Approac	h	563	2.0	0.437	14.7	LOS B	4.1	103.3	0.86	0.94	28.3
West: Al	bany W	est									
5L	Ĺ	95	2.0	0.675	16.1	LOS B	6.1	156.0	0.74	1.05	28.8
2T	Т	507	2.0	0.675	8.6	LOS A	6.1	156.0	0.74	0.80	30.9
2R	R	363	2.0	0.476	9.1	LOS A	3.1	78.9	0.63	0.77	31.0
Approac	:h	964	2.0	0.676	9.5	LOS B	6.1	156.0	0.70	0.81	30.7
All Vehic	cles	2926	2.0	0.676	11.6	LOS B	6.1	156.0	0.77	0.87	29.7

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).

Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

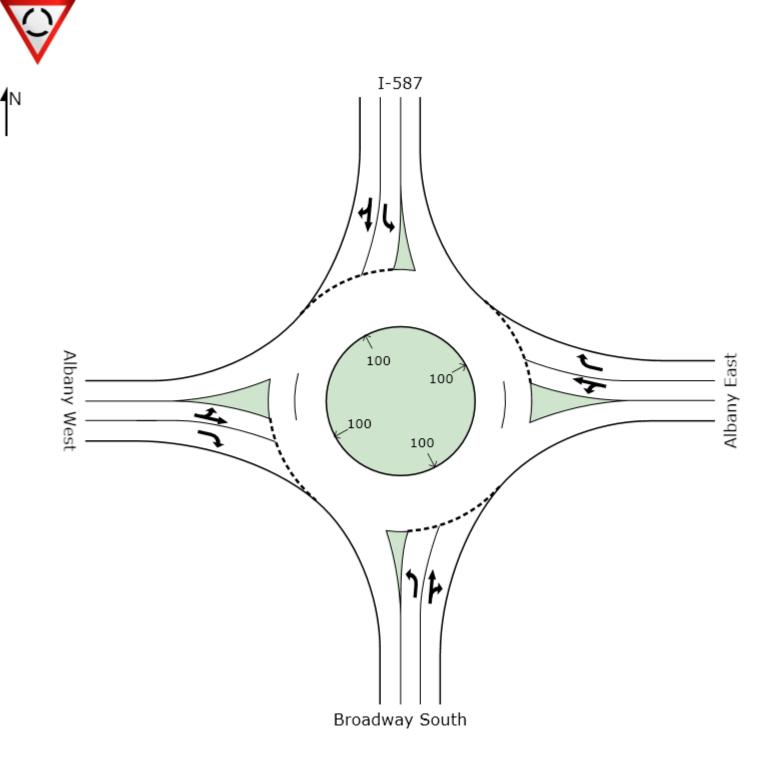
Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

Processed: Tuesday, October 05, 2010 7:49:33 AM SIDRA INTERSECTION 5.0.2.1437 Project: \\.psf\Project files\Kingston, NY\Analyses\Kingston Albany.sip 8000067, ALTERNATE STREET DESIGN PA, SINGLE





Albany and I-587 Option 2 right only and through left from I-587 PM Peak Roundabout

Movem	ent Pe	rformance - V	/ehicles								
		Demand		Deg.	Average	Level of	95% Back of		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 11 0		veh/h	%	v/c	sec	_	veh	ft	_	per veh	mph
South: B		,									
3L	L	242	2.0	0.534	24.6	LOS C	5.4	137.3	0.96	1.08	24.0
8T	Т	293	2.0	0.607	17.8	LOS B	7.3	184.8	1.00	1.13	26.2
8R	R	39	2.0	0.603	19.3	LOS B	7.3	184.8	1.00	1.13	26.1
Approac	h	574	2.0	0.607	20.8	LOS C	7.3	184.8	0.98	1.11	25.2
East: Alb	bany Ea	st									
1L	L	60	2.0	0.778	19.0	LOS B	8.8	222.6	0.88	1.13	27.3
6T	Т	548	2.0	0.778	11.9	LOS B	8.8	222.6	0.88	1.06	29.6
6R	R	301	2.0	0.488	10.8	LOS B	3.4	85.4	0.71	0.90	30.4
Approac	h	909	2.0	0.777	12.0	LOS B	8.8	222.6	0.83	1.01	29.6
North: I-	587										
7L	L	268	2.0	0.816	32.2	LOS C	14.0	355.3	1.00	1.32	21.7
4T	Т	248	2.0	0.815	25.0	LOS C	14.0	355.3	1.00	1.31	22.6
4R	R	81	2.0	0.230	15.9	LOS B	1.6	40.0	0.82	0.92	27.3
Approac	h	597	2.0	0.815	27.0	LOS C	14.0	355.3	0.98	1.26	22.7
West: Al	bany W	est									
5L	L	100	2.0	0.808	24.3	LOS C	13.8	351.3	1.00	1.17	24.9
2T	Т	537	2.0	0.807	17.1	LOS B	13.8	351.3	1.00	1.17	26.5
2R	R	385	2.0	0.593	14.1	LOS B	6.5	164.1	0.90	1.01	28.3
Approac	h	1022	2.0	0.807	16.7	LOS C	13.8	351.3	0.96	1.11	27.0
All Vehic	les	3102	2.0	0.815	18.1	LOS B	14.0	355.3	0.93	1.11	26.3

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).

Level of Service (Worst Movement): LOS C. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

Flow Scale Analysis Objective: Practical Capacity (v/c ratio = xp). (Results for Flow Scale = 106.0 % largest for any movement)

Processed: Tuesday, October 05, 2010 7:55:38 AM SIDRA INTERSECTION 5.0.2.1437

Copyright © 2000-2010 Akcelik & Associates Pty Ltd www.sidrasolutions.com



Project: \\.psf\Project files\Kingston, NY\Analyses\Kingston Albany.sip 8000067, ALTERNATE STREET DESIGN PA, SINGLE



#### Appendix C: Route 28/I-87/I-587/Washington Avenue Roundabout

Michael Wallwork is one of North America's top roundabout designers and he is an author of several roundabout design guidelines. He helped design and analyze the roundabout recommended in this study. He also heard about the problems associated with the existing roundabout at I-87. A large part of his practice is fixing early generation roundabouts. Consequently, Mr. Wallwork conducted a couple of site visits to the existing roundabout, observed its operation, and recognized its design shortcomings and summarized them below.

Many comments were made regarding the existing roundabout at I-87; therefore, the roundabout was driven several times, the operation observed from various points and the crash performance discussed with Mr. Charlie Schaller, the County's Safety Engineer. There are approximately 61 crashes a year, most of which are caused by drivers making illegal left turns. My observations found that this roundabout has by far the most illegal left turns I have ever seen at a roundabout. When large trucks turn, their trailers take a straighter path than the truck cab creating a wide swept path. Because of non-standard design, the truck apron is too high and too narrow to accommodate the swept paths of large trucks, forcing truck drivers to make illegal left turns; i.e. turning left from the outside approach lane.

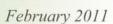
By rebuilding the truck apron so that is only three inches high and sufficiently wide to accommodate the swept paths of large trucks, say 10 feet or so wide, these trucks could then make legal left turns from the left most lane. After this change lane arrows could be added to the approach lanes to direct and educate all drivers as to which lanes to use when entering and turning at this roundabout. Additionally, the roundabout could be refined further by lowering the berm in the center of the roundabout to a maximum of four feet, and redesigning the splitter islands. A proper redesign of the splitter islands could also lower vehicle entering and exit speeds. Consideration should also be given to the design and location of the trail crossing on Washington Avenue south of the roundabout, where is across the widest section of the road and where three lanes merge.

February 2011



Six inch truck apron too high to mount effectively







Trucks make illegal left turns from the outside lane



High berm at center of roundabout restricts sight lines

Based on the 2010 NYSDOT Highway Design Manual, general objectives for intersection design are:

- To provide adequate sight distances.
- To minimize points of conflict.
- To simplify conflict areas.
- To limit conflict frequency.
- To minimize severity of conflicts.
- To minimize delay.
- To provide acceptable capacity for the design year.

Roundabouts are frequently able to address the above objectives better than other intersection types in both urban and rural environments and on high- and low-speed highways. Thus, when a project includes reconstructing or constructing new intersections, a roundabout alternative is to be analyzed to determine if it is a feasible solution based on site constraints, including ROW, environmental factors, and other design constraints. Exceptions to this requirement are where the intersection:

- Has no current or anticipated safety, capacity, or other operational problems.
- Is within a well working coordinated signal system in a lowspeed (<80 km/h) urban environment with acceptable accident histories.
- Is where signals will be installed solely for emergency vehicle preemption.
- Has steep terrain that makes providing an area, graded at 5% or less for the circulating roadways, infeasible.
- Has been deemed unsuitable for a roundabout by the Roundabout Design Unit.

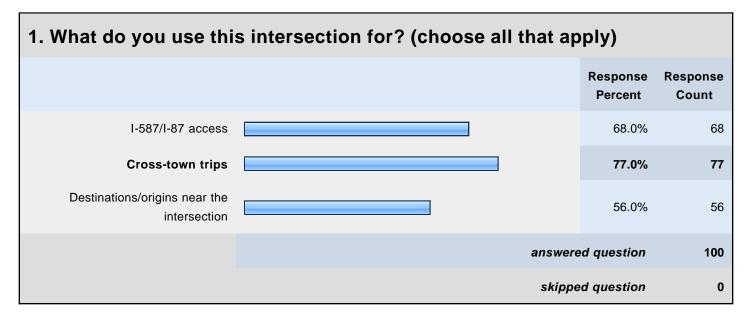
When the analysis shows that a roundabout is a feasible alternative, it should be considered the Department's <u>preferred</u> <u>alternative</u> due to the proven substantial safety benefits and other operational benefits.

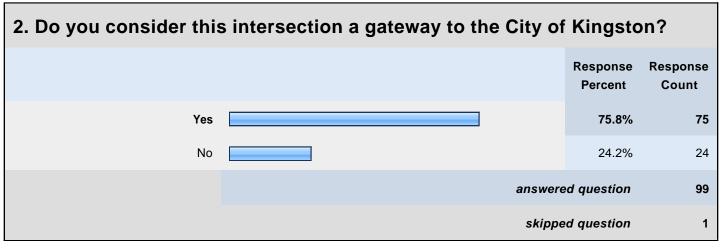


February 2011

in

Appendix D: Public Survey

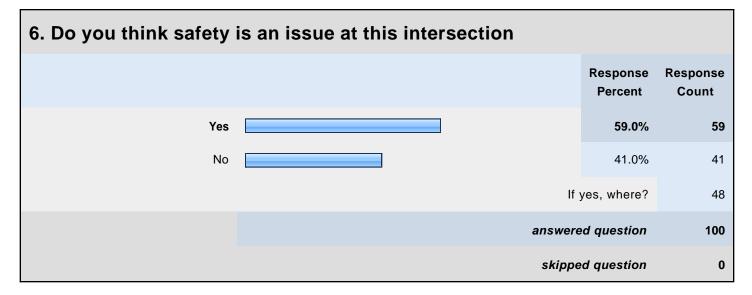




3. Who do you think this intersection should primarily serve?								
			Response Percent	Response Count				
Local residents & businesses			8.0%	8				
Through traffic			8.0%	8				
Serve both equally			84.0%	84				
		answere	ed question	100				
		skippe	ed question	0				

4. How much of an improvement do you think this intersection needs?								
		Response Percent	Response Count					
None, just maintenance		19.0%	19					
Some improvements and maintenance		36.0%	36					
Complete redesign/rebuild		45.0%	45					
	answere	ed question	100					
	skippe	ed question	0					

5. Do you think that traffic congestion is a problem at this intersection							
		Response Percent	Response Count				
Yes		45.0%	45				
No		14.0%	14				
Only periodically		41.0%	41				
	answere	ed question	100				
	skippe	ed question	0				



7. What do you think causes traffic congestion at this intersection? (choose all that apply)			
		Response Percent	Response Count
Too much traffic		48.4%	44
Intersection design		62.6%	57
Traffic light timing		47.3%	43
	Other (ple	ase specify)	13
	answere	ed question	91
	skippe	ed question	9

8. How would you expect traffic to behave following the intersection improvement?				
			Response Percent	Response Count
Slow with predictable delay			38.0%	35
Fast with minimal delay		l	62.0%	57
		answere	d question	92
		skippe	d question	8

# 9. During what hours (if any) would you avoid driving through this intersection?

		Response Percent	Response Count
6am		2.6%	2
7		19.7%	15
8		57.9%	44
9		50.0%	38
10		7.9%	6
11		6.6%	5
12pm		30.3%	23
1		19.7%	15
2		6.6%	5
3		25.0%	19
4		52.6%	40
5		86.8%	66
6		44.7%	34
7		5.3%	4
8pm		1.3%	1
	answere	ed question	76
	skippe	ed question	24

#### 10. If you had to pick just one type of improvement to this intersection, what should it accomplish? Response Response Percent Count **Relieve congestion** 49.5% 49 Improve appearance 19.2% 19 Maximize safety 15.2% 15 2.0% Accommodate pedestrians 2 Bicycles and transit improvements 5.1% 5 Economic development 9.1% 9 answered question 99 skipped question 1

11. Would you be more likely to walk or bike in the vicinity of this intersection if substantial improvements were made to the sidewalks, crosswalks, bike lanes, and streetscape?

		Response Percent	Response Count
Yes	]	42.4%	42
No	]	42.4%	42
Maybe		15.2%	15
	answere	ed question	99
	skippe	ed question	1

12. Is this intersection adequately illuminated at night?				
		Response Percent	Response Count	
Yes		71.6%	68	
No		28.4%	27	
	answere	ed question	95	
	skippe	ed question	5	

13. Is the directional signage at this intersection sufficient? For example, given the existing signage, is it clear what lane to use?				
			Response Percent	Response Count
Yes			58.0%	58
No			42.0%	42
		answere	ed question	100
		skippe	ed question	0

14. Do you think that there is adequate parking for the businesses in this area?

		Response Percent	Response Count
Yes		37.6%	35
Νο		62.4%	58
	answer	ed question	93
	skipp	ed question	7

15. There are many small parcels of green space at this intersection now, if these spaces could be combined, do you think that a small park would be appropriate at this intersection?			
	Response Percent	Response Count	
Yes	38.0%	38	
Νο	62.0%	62	
	answered question	100	
	skipped question	0	



February 2011

Appendix E: Advisory Committee Presentation



• Costs

